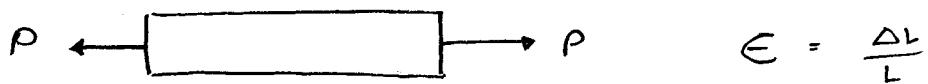
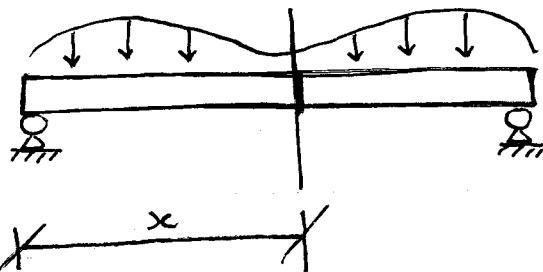
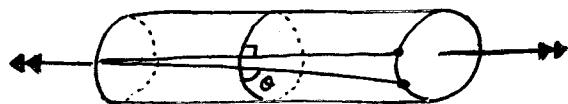


Ch. 12 Deflection of Beams and Shafts



$$\epsilon = \frac{\Delta L}{L}$$



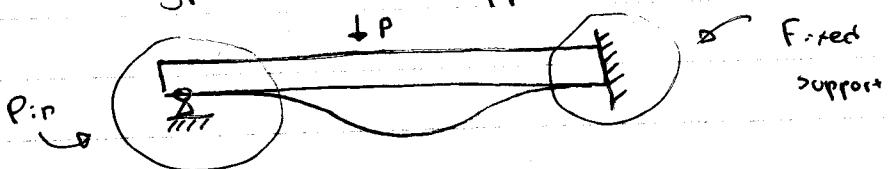
$$\sigma = -\frac{My}{I}$$

12.1 The Elastic Curve

The deflection diagram of the longitudinal axis that through the centroid of each cross-section area of the beam is called the elastic curve.

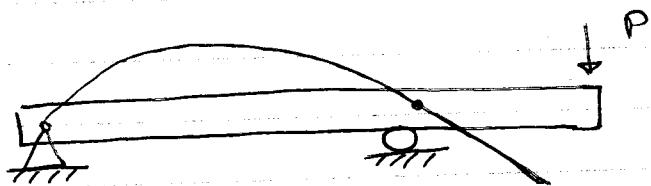
Sketch the elastic curve

- 1) The slope/disp variation at different types of supports.



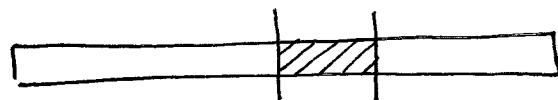
Pin: Resisting a Force, restrict disp.

Fixed: Resisting a Force and a moment
Restrict displacement and rotation

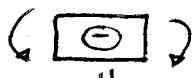


(2)

2) The relationship between moments and elastic curve.

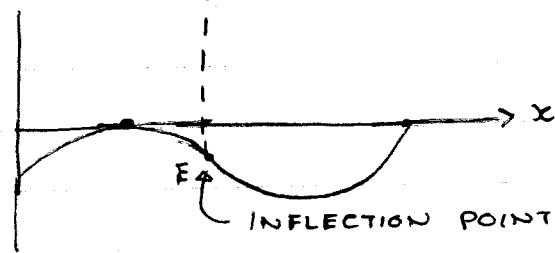
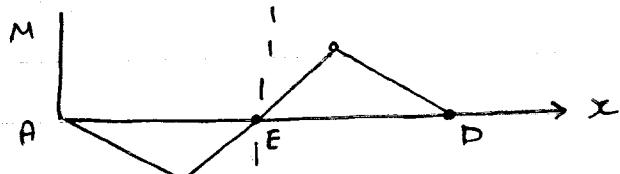
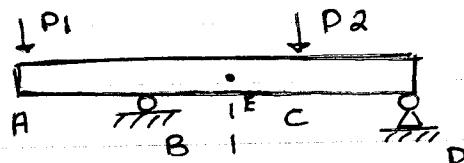


↳ concave upwards



↳ concave downwards

Example:

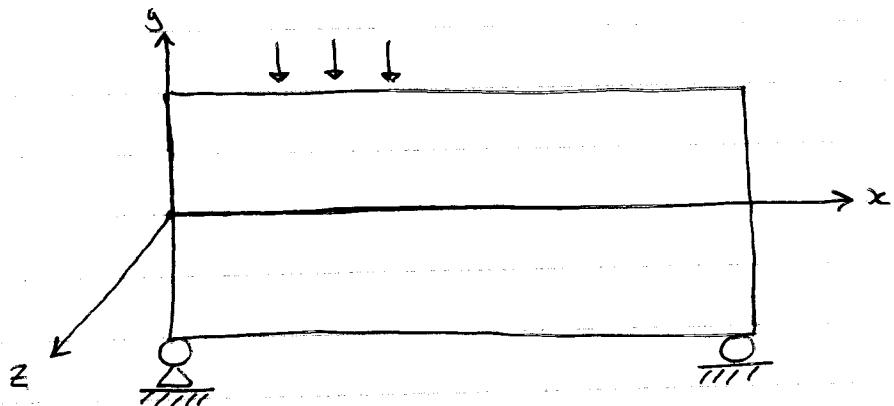


Bending Deformation:

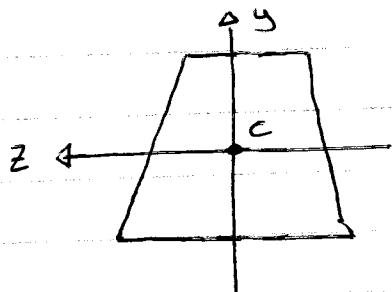
#1: The longitudinal axis (x) which lies within the neutral surface does not experience

any change in length

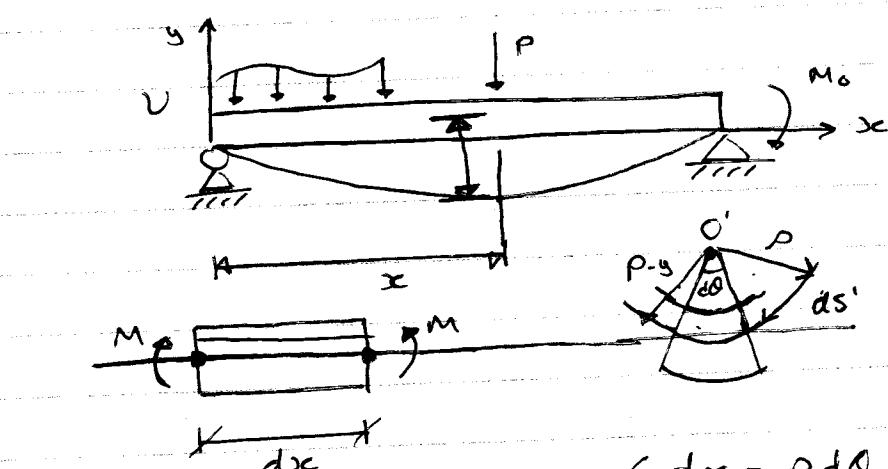
#2: All cross-sections of the beam remain plane and perpendicular to the longitudinal axis during the deformation.



xg : plane of symmetry



Moment - Curvature relationship



$$\left\{ \begin{array}{l} dx = \rho d\theta \\ ds' = (P-g) d\theta \end{array} \right. = \frac{(P-g)d\theta - Pd\theta}{\rho d\theta}$$

For Normal Strain

$$\epsilon = \frac{ds - dx}{dx} = \frac{(\rho y) d\theta - \rho d\theta}{\rho d\theta}$$

$$\boxed{\epsilon = -\frac{y}{\rho}}$$

Hooke's Law:

$$\sigma = E\epsilon = \frac{-Ey}{\rho} *$$

The Flexure Formula:

$$\Rightarrow \frac{-Ey}{\rho} = \frac{-My}{I} *$$

$$\boxed{\Rightarrow \frac{1}{\rho} = \frac{M}{EI}}$$

Flexural Rigidity: EI

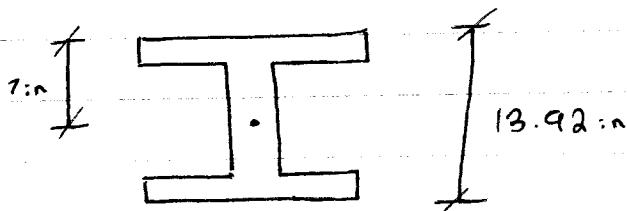
Special Case: $M = \text{const}$ (pure bending)

ρ will be a constant as well.

↳ circular arc.

A beam, A36 Steel, W14x53

$$\left\{ \begin{array}{l} E_{st} = 29(10^3) \text{ ksi} \\ S_y = 36 \text{ ksi} \end{array} \right.$$

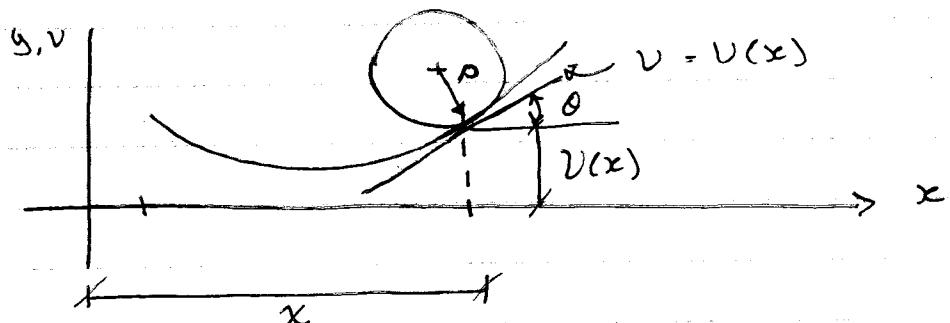


$$\text{If } \sigma_{\max} = \sigma_y$$

$$y_p = -\frac{E_s}{\sigma_{\max}} = -\frac{\sigma_{\max}}{E_s}$$

$$|P| = \left| \frac{E_s}{S_{\max}} \right| = \frac{(29 \times 10^3)(7)}{36} = \boxed{5639 \text{ in}}$$

12.2 Slope and Displacement by Integration



$$\frac{1}{P} = \frac{M}{EI}$$

$$\frac{1}{P} = \frac{d^2v/dx^2}{[1 + (dv/dx)^2]^{3/2}}$$

Small Deformation:

$v(x)$ is small, $|dv/dx| \ll 1$ notation represents "much" small

$$\frac{dv}{dx} = \tan \theta \approx \theta$$

$$\theta = \frac{dv}{dx}$$

- * Slope angle
- * Rotation of the cross-section

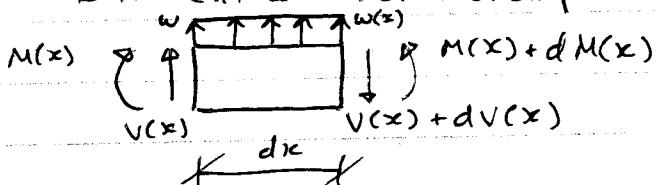


Small Deformation

$$\frac{1}{P} = \frac{d^2v}{dx^2}$$

$$\Rightarrow \frac{d^2v}{dx^2} = \frac{M}{EI}$$

Differential relationship:



(6)

$$\sum F = 0 : V(x) + w(x)dx - V(x) - dV(x) = 0$$

$$\boxed{\frac{dV}{dx} = w}$$

$$\sum M = 0 : \boxed{\frac{dM}{dx} = V}$$

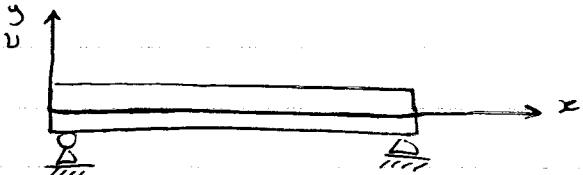
Since $M(x) = EI \frac{d^2V}{dx^2}$

$$\Rightarrow \boxed{\frac{d}{dx} \left(EI \frac{d^2V}{dx^2} \right) = V}$$

$$\Rightarrow \boxed{\frac{d^2}{dx^2} \left(EI \frac{d^2V}{dx^2} \right) = w}$$

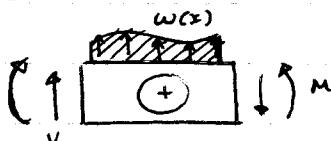
MAR. 8 /17

12.2 Slope and displacement by integration



$$(1) E = \frac{d^2 v(x)}{dx^2} = M(x)$$

Sign convention:

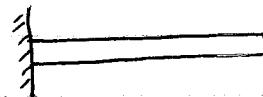


$$(2) \frac{d}{dx} \left(EI \frac{d^2 v(x)}{dx^2} \right) = V(x)$$

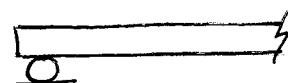
$$(3) \frac{d^2}{dx^2} \left(EI \frac{d^2 v(x)}{dx^2} \right) = W(x)$$

Domain: $0 \leq x \leq L$

Boundary Conditions



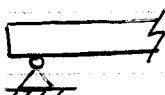
Roller:



Deflection

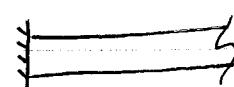
$$v = 0, M = 0$$

Pin:



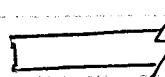
$$v = 0, M = 0$$

Fixed:



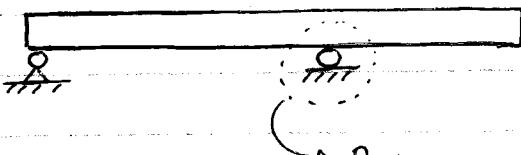
$$v = 0, M = 0$$

Free:



$$v = 0, M = 0$$

Shear force



$$v = 0$$

Roller

Pin



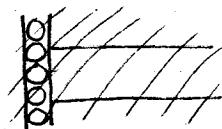
$$v = 0$$

(2)

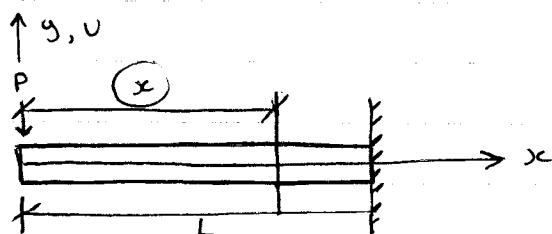


Internal pin hinge

$$M = 0$$

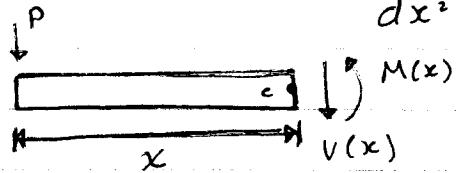


Example:



Determine the elastic curve, $EI = \text{const}$

Solution: $EI \frac{d^2v}{dx^2} = M$



$$\sum M_c = 0 :$$

$$M(x) + Px = 0$$

$$M(x) = -Px$$

Elastic Curve

$$EI \frac{d^2v}{dx^2} = -Px$$

$$EI \frac{dv}{dx} = -\frac{P}{2}x^2 + C_1$$

$$* EIv = -\frac{Px^3}{6} + C_1x + C_2$$

Boundary conditions

$$\begin{cases} x = L, v = 0 \\ x = L, \theta = \frac{dv}{dx} = 0 \end{cases}$$

$$\Rightarrow -\frac{PL^3}{6} + [C_1L + C_2] = 0$$

$$\Rightarrow -\frac{PL^2}{2} + [C_1] = 0$$

$$\Rightarrow C_1 = \frac{PL^2}{2}, C_2 = -\frac{1}{3}PL^3$$

$$EIv = -\frac{Px^3}{6} + \frac{PL^2}{2}x - \frac{1}{3}PL^3$$

2

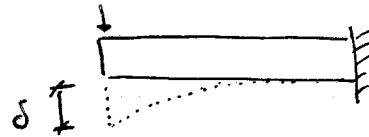
(3)

$$\Rightarrow V = \frac{-P}{6EI} (x^3 - 3L^2x + 2L^3) *$$

and $\theta = \frac{dv}{dx} = \frac{-P}{2EI} (x^2 - L^2)$

max deflectionit occurs at $x = 0$

$$v_{\max} = -\frac{PL^3}{3EI}$$

and the rotation at $x = 0$ is

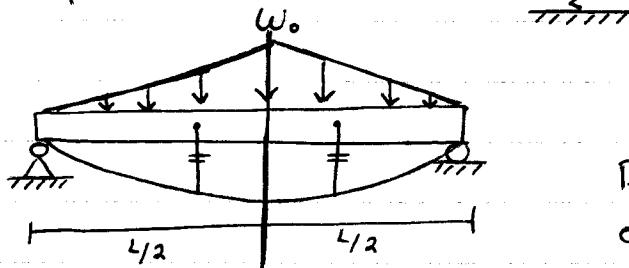
$$\theta|_{x=0} = \frac{PL^2}{2EI}$$

At the Free end

$$\delta = \frac{PL^3}{3EI} \Rightarrow P = \frac{3EI}{L^3} \cdot \delta$$



Example:



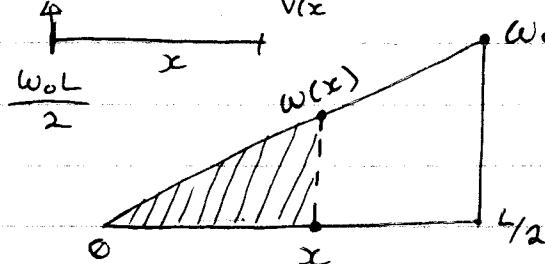
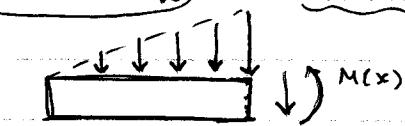
$$EI = \text{const}$$

Determine the max deflection of the beam.

$$\text{Solution: } EI \frac{d^2v}{dx^2} = M(x)$$

$$0 \leq x \leq \frac{L}{2} :$$

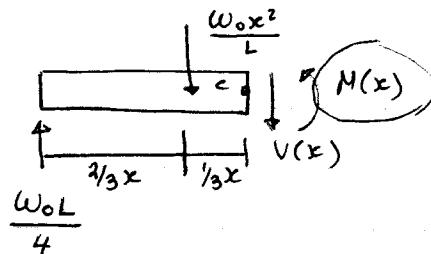
$$\frac{1}{2}w_0 \times \frac{L}{2} \times 2 \times \frac{1}{2} = \frac{w_0 L}{4}$$



$$\frac{w(x)}{w_0} = \frac{x}{L/2}$$

$$w(x) = \frac{2w_0 x}{L}$$

$$\frac{1}{2}x \cdot w(x) = \frac{1}{2}x \cdot \frac{2w_0x}{L} = \frac{w_0x^2}{L}$$



$$\sum M_e = 0 :$$

$$0 = M(x) + \frac{w_0x^2}{L} \cdot \frac{1}{3}x - \frac{w_0L \cdot x}{4}$$

$$\Rightarrow M(x) = -\frac{w_0x^3}{3L} + \frac{w_0Lx}{4}$$

Elastic Curve

$$EI \frac{d^2V}{dx^2} = -\frac{w_0x^3}{3L} + \frac{w_0Lx}{4}$$

$$EI \frac{dV}{dx} = -\frac{w_0x^4}{12L} + \frac{w_0Lx^2}{8} + C_1$$

$$EI V = -\frac{w_0x^5}{60L} + \frac{w_0Lx^3}{24} + C_1x + C_2$$

Boundary Conditions

$$\text{At } x=0, V=0$$

$$0 = 0 + C_2 \Rightarrow C_2 = 0$$

$$\text{At } x=L/2, \theta=0$$

$$0 = -\frac{w_0}{12L} \left(\frac{L}{2}\right)^4 + \frac{w_0L}{8} \left(\frac{L}{2}\right)^2 + C_1$$

$$\Rightarrow C_1 = -\frac{5w_0L^3}{192}$$

Deflection

$$EI V = -\frac{w_0x^5}{60L} + \frac{w_0Lx^3}{24} - \frac{5w_0Lx}{192}$$

The max deflection occurs when $x=L/2$

$$V_{\max} = \frac{1}{EI} \left[-\frac{w_0}{60L} \left(\frac{L}{2}\right)^5 + \frac{w_0L}{24} \left(\frac{L}{2}\right)^3 - \frac{5w_0L^3}{192} \left(\frac{L}{2}\right) \right]$$

$$= -\frac{w_0L^4}{120EI}$$