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Oct. 2/18

Chapter 6 : Torsion

Displacement $\vec{u} = u\hat{i} + v\hat{j} + w\hat{k}$

Strains : $\epsilon_{xx} = \frac{\partial u}{\partial x}$

$\epsilon_{yy} = \frac{\partial v}{\partial y}$

$\epsilon_{zz} = \frac{\partial w}{\partial z}$

$\epsilon_{yz} = (\frac{1}{2})\left(\frac{\partial v}{\partial z} + \frac{\partial w}{\partial y}\right)$

$\epsilon_{zx} = (\frac{1}{2})\left(\frac{\partial w}{\partial x} + \frac{\partial u}{\partial z}\right)$

$\epsilon_{xy} = (\frac{1}{2})\left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x}\right)$

Stresses : $\sigma_{xx}, \sigma_{yy}, \sigma_{zz}, \tau_{yz}, \tau_{zx}, \tau_{xy}$

Equilibrium

$$\left\{ \begin{array}{l} \frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{yy}}{\partial y} + \dots \\ \vdots \end{array} \right\}$$

Hooke's Law for isotropic :

$\sigma_{xx} = \lambda e + 2G\epsilon_{xx}$

$\sigma_{yy} = \lambda e + 2G\epsilon_{yy}$

$\sigma_{zz} = \lambda e + 2G\epsilon_{zz}$

$\tau_{yz} = 2G\epsilon_{yz}$

$\tau_{zx} = 2G\epsilon_{zx}$

$\tau_{xy} = 2G\epsilon_{xy}$

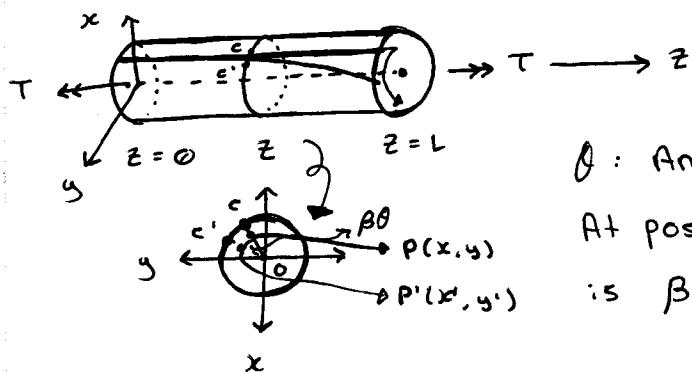
15 equations for 15 unknowns

boundary conditions :

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6.1 Torsion of a prismatic bar of circular

cross section

 ϕ : Angle of twist per unit lengthAt position Z , the angle of twist is $\beta = \theta_z$

Displacement of a point on plane Z :

$$P(x, y) \rightarrow P'(x', y') :$$

$$\therefore \begin{cases} u(x, y, z) = x' - x \\ v(x, y, z) = y' - y \\ w(x, y, z) = \phi \end{cases}$$

Define $OP = r$,

$$x = r \cos \phi \quad y = r \sin \phi$$

$$x' = r \cos(\phi + \beta) \quad y' = r \sin(\phi + \beta)$$

$$\therefore x' = r \cos \phi \cos \beta - r \sin \phi \sin \beta \\ = r \cos \theta_z - r \sin \theta_z$$

$$y' = r \sin \phi \cos \beta - r \cos \phi \sin \beta \\ = r \cos \theta_z + r \sin \theta_z$$

Small deformation, ϕ is very small, θ_z is small,

$$\sin \theta_z \approx \theta_z \quad \cos \theta_z \approx 1$$

$$\Rightarrow x' = x - \theta_y z$$

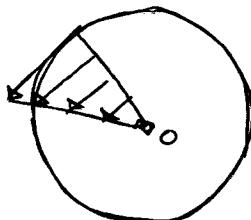
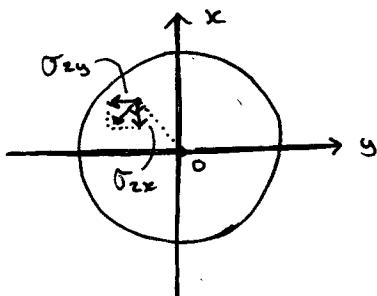
$$y' = y + \theta_x z$$

$$\therefore \begin{cases} u = x' - x = -\theta_y z \\ v = y' - y = \theta_x z \\ w = \phi \end{cases}$$

$$\begin{aligned} \text{strain } \epsilon_{xx} &= \frac{\partial u}{\partial x} = 0, \quad \epsilon_{yy} = \frac{\partial v}{\partial y} = 0, \quad \epsilon_{zz} = 0 \\ \epsilon_{xy} &= \left(\frac{1}{2}\right)\left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x}\right) = \frac{1}{2}(-\theta_z + \theta_z) = 0 \\ \epsilon_{xz} &= \left(\frac{1}{2}\right)\left(\frac{\partial u}{\partial z}\right) = -\frac{1}{2}\theta_y \\ \epsilon_{yz} &= \left(\frac{1}{2}\right)\left(\frac{\partial v}{\partial z}\right) = \frac{1}{2}\theta_x \end{aligned}$$

Hooke's Law :

$$\begin{cases} \sigma_{xx} = \sigma_{yy} = \sigma_{zz} = \sigma_{xy} = 0 \\ \sigma_{xz} = 2G\epsilon_{xz} = -G\theta_y \\ \sigma_{yz} = 2G\epsilon_{yz} = G\theta_x \end{cases}$$



resultant of stress component:

$$\begin{aligned} \tau &= \sqrt{\sigma_{xz}^2 + \sigma_{yz}^2} \\ &= \sqrt{(-G\theta_y)^2 + (G\theta_x)^2} \\ &= G\theta \sqrt{x^2 + y^2} \\ \boxed{\tau = G\theta r} \end{aligned}$$

From statics :

$$\begin{aligned} T &= \iint_A \tau r \, dr \, d\theta \\ &= \int_0^{2\pi} d\theta \int_0^r G\theta r \cdot r \cdot dr \\ &= 2\pi G\theta \cdot \left(\frac{1}{4}\right) b^4 \\ T &= G\theta \cdot \frac{\pi}{2} b^4 \end{aligned}$$

Define $J = \frac{\pi}{2} b^4$ (Polar moment of inertia)

$$\Rightarrow \boxed{T = G\theta J = GJ\theta}$$

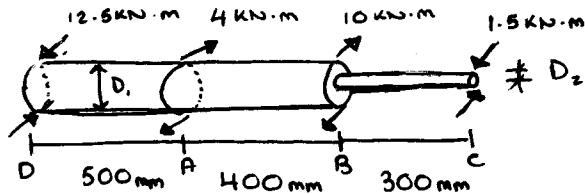
\therefore Angle of twist per unit length

$$\boxed{\theta = T/GJ}$$

Stress :

$$\boxed{\tau = G\theta r = (T/J) \cdot r}$$

Example 6.4 - from textbook



$$D_1 = 100 \text{ mm}$$

$$D_2 = 50 \text{ mm}$$

$$G = 77.5 \text{ GPa}$$

Find 1° the max shear stress

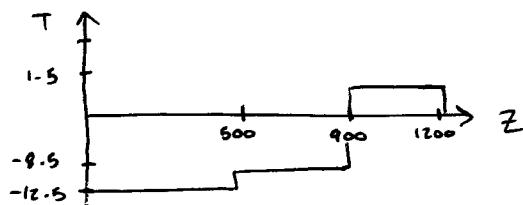
2° the angle of twist of sections A, B, C,
relative to section D.

Solution :



$$\underline{\underline{DA}} \quad D \xrightarrow{-12.5} \xrightarrow{T_{DA}} \quad \sum T = 0, \quad T_{DA} + 12.5 = 0 \\ T_{DA} = -12.5 \text{ kN}\cdot\text{m}$$

$$\underline{\underline{AB}} \quad D \xrightarrow{-12.5} \xrightarrow{4} \xrightarrow{T} \quad \sum T = 0 \quad \therefore T_{AB} = 8.5 \text{ kN}\cdot\text{m} \\ \xrightarrow{\quad \quad \quad} \therefore T_{BC} = 1.5 \text{ kN}\cdot\text{m}$$



$$J_{DA} = \frac{\pi}{2} b^4 = \frac{\pi}{2} \left(\frac{100}{2}\right)^4 = 9.8175 (10^{-6}) \text{ m}^4$$

$$J_{AB} = J_{DA} = 9.8175 (10^{-6}) \text{ m}^4$$

$$J_{BC} = \frac{\pi}{2} b^4 = \frac{\pi}{2} \left(\frac{50}{2}\right)^4 = 6.1359 (10^{-6}) \text{ m}^4$$

$$\underline{\underline{AD}} \quad \underline{\underline{Z}}_{max} = \left(\frac{T}{J}\right) \Gamma_{max} = \frac{12.5 (10^3)}{9.8175 (10^{-6})} \cdot \frac{(100)(10^{-3})}{2} \\ \Rightarrow 63.66 (10^6) \text{ Pa}$$

$$63.66 \text{ MPa}$$

$$\underline{\underline{BC}} \quad \underline{\underline{Z}}_{max} = \left(\frac{T}{J}\right) \Gamma_{max} = \frac{1.5 (10^3)}{6.1359 (10^{-6})} \cdot \frac{50}{2} (10^{-3}) \\ = 61.12 \text{ MPa}$$

$$\therefore \underline{\underline{Z}}_{max} = 63.66 \text{ MPa}$$

MOM

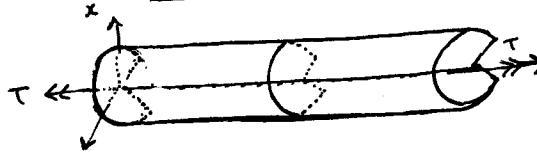
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$$\begin{aligned}\beta_{A/D} &= \frac{\text{AD}}{\text{AB}} \cdot \frac{\theta_{AD}}{\theta_{AB}} \cdot \frac{\text{rad}}{\pi} \\ &= \text{AD} \cdot \frac{T}{GJ} \\ &= (500)(10^{-3}) \cdot \frac{(-12.5)(10^3)}{(77.5)(10^4)(10^{-6})} \Rightarrow \beta_{A/D} = -0.00821 \text{ rad}\end{aligned}$$

$$\begin{aligned}\beta_{B/A} &= AB \cdot \frac{\theta_{AB}}{\theta_{AD}} \\ &= AB \cdot \frac{T}{GJ} \\ &\Rightarrow -0.00447 \text{ rad}\end{aligned}$$

$$\beta_{C/B} = BC \cdot \frac{\theta_{CB}}{\theta_{AB}} = ?$$

6.2 Saint-Venant's Semi-inverse Method



θ : angle of twist per unit length
(constant)

$$\text{Assumption: } u(x, y, z) = -\theta y z$$

$$v(x, y, z) = \theta x z$$

$$w(x, y, z) = \theta \psi(x, y)$$

warping

$$\text{Strains: } \epsilon_{xx} = \frac{\partial u}{\partial x} = 0, \quad \epsilon_{yy} = \frac{\partial v}{\partial y} = 0, \quad \epsilon_{zz} = \frac{\partial w}{\partial z} = 0$$

$$\epsilon_{xy} = \frac{1}{2} \left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) = \frac{1}{2} (-\theta z + \theta z) = 0$$

$$\epsilon_{xz} = \frac{1}{2} \left(\frac{\partial u}{\partial z} + \frac{\partial w}{\partial x} \right) = \frac{1}{2} (\theta \frac{\partial \psi}{\partial x} - \theta y)$$

$$\epsilon_{yz} = \frac{1}{2} \left(\frac{\partial v}{\partial z} + \frac{\partial w}{\partial y} \right) = \frac{1}{2} (\theta \frac{\partial \psi}{\partial y} + \theta x)$$

$$\text{Stress: } \sigma_{xx} = \sigma_{yy} = \sigma_{zz} = 0$$

$$\begin{cases} \sigma_{xz} = 2G \epsilon_{xz} = G \theta (\frac{\partial \psi}{\partial x} - y) \\ \sigma_{yz} = 2G \epsilon_{yz} = G \theta (\frac{\partial \psi}{\partial y} + x) \end{cases}$$

Equilibrium eq's:

$$\cancel{\frac{\partial \sigma_{xz}}{\partial x}} + \cancel{\frac{\partial \sigma_{yz}}{\partial y}} + \cancel{\frac{\partial \sigma_{xx}}{\partial z}} = 0$$

$$\cancel{\frac{\partial \sigma_{yz}}{\partial x}} + \cancel{\frac{\partial \sigma_{yy}}{\partial y}} + \cancel{\frac{\partial \sigma_{yy}}{\partial z}} = 0$$

$$\cancel{\frac{\partial \sigma_{xz}}{\partial x}} + \cancel{\frac{\partial \sigma_{zy}}{\partial y}} + \cancel{\frac{\partial \sigma_{zz}}{\partial z}} = 0$$

$$\Rightarrow \boxed{\frac{\partial \sigma_{xz}}{\partial x} + \frac{\partial \sigma_{yz}}{\partial y} = 0}$$

$$\Rightarrow \sigma_{xz} = \frac{\partial \phi}{\partial y} \quad ; \quad \sigma_{yz} = -\frac{\partial \phi}{\partial x}$$

$\phi = \phi(x, y)$: stress function

$$\text{Since } \epsilon_{xz} = \frac{1}{2} \theta (\frac{\partial \psi}{\partial x} - y)$$

$$\epsilon_{yz} = \frac{1}{2} \theta (\frac{\partial \psi}{\partial y} + x)$$

$$\Rightarrow \frac{\partial \epsilon_{xz}}{\partial y} = \frac{1}{2} \theta \left(\frac{\partial^2 \psi}{\partial y \partial x} - 1 \right)$$

$$\frac{\partial \epsilon_{yz}}{\partial x} = \frac{1}{2} \theta \left(\frac{\partial^2 \psi}{\partial x \partial y} + 1 \right)$$

$$\Rightarrow \boxed{\frac{\partial \epsilon_{yz}}{\partial x} - \frac{\partial \epsilon_{xz}}{\partial y} = \theta}$$

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$$\Rightarrow \frac{\partial}{\partial x} (2G \epsilon_{yz}) - \frac{\partial}{\partial y} (2G \epsilon_{xz}) = 2G\theta$$

$$\Rightarrow \frac{\partial \sigma_{yz}}{\partial x} - \frac{\partial \sigma_{xz}}{\partial y} = 2G\theta$$

Using stress function:

$$-\frac{\partial^2 \phi}{\partial x^2} - \frac{\partial^2 \phi}{\partial y^2} = 2G\theta$$

$$\Rightarrow \boxed{\frac{\partial^2 \phi}{\partial x^2} + \frac{\partial^2 \phi}{\partial y^2} = -2G\theta}$$

Poisson's
Equation

Boundary conditions

$$\vec{n} = (l, m, \phi)$$

$$\left\{ \begin{array}{l} \sigma_{xx} l + \sigma_{xy} m + \sigma_{xz} \phi = 0 \\ \sigma_{yx} l + \sigma_{yy} m + \sigma_{yz} \phi = 0 \\ \sigma_{zx} l + \sigma_{zy} m + \sigma_{zz} \phi = 0 \end{array} \right. \quad \text{traction-free}$$

$$\Rightarrow \sigma_{xz} l + \sigma_{yz} m = 0$$

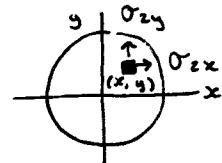
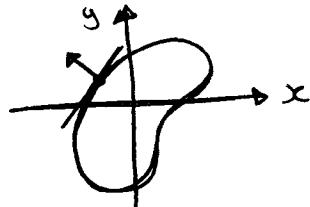
$$\Rightarrow \frac{d\phi}{ds} = 0$$

$$\Rightarrow \phi = \text{const along the boundary}$$

$$\text{Solid cross-section: } \phi = 0$$

Resultant of Stress on XY-plane:

$$\iint_A \sigma_{xx} dx dy = F_x = 0$$



$$\iint_A \sigma_{xy} dx dy = F_y = 0$$

$$\boxed{\iint_A (\sigma_{xy} x dx dy - \sigma_{xz} y dx dy) = T}$$

$$\Rightarrow \iint_A \left(-\frac{\partial \phi}{\partial x} \cdot x - \frac{\partial \phi}{\partial y} \cdot y \right) dx dy = T$$

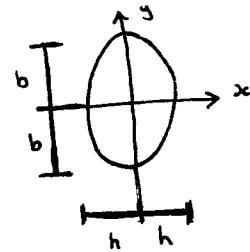
$$\Rightarrow \boxed{T = 2 \iint_A \phi dx dy}$$

6.3 Linear Elastic Solution

6.3.1 Elliptical cross section

Boundary curve:

$$\frac{x^2}{h^2} + \frac{y^2}{b^2} - 1 = 0$$



Assume:

$$\phi(x, y) = B \left(\frac{x^2}{h^2} + \frac{y^2}{b^2} - 1 \right)$$

Then $\phi = 0$ on the boundary

and $\frac{\partial^2 \phi}{\partial x^2} = \frac{2B}{h^2}$ $\frac{\partial^2 \phi}{\partial y^2} = \frac{2B}{b^2}$

$$\therefore \frac{\partial^2 \phi}{\partial x^2} + \frac{\partial^2 \phi}{\partial y^2} = 2B \left(\frac{1}{h^2} + \frac{1}{b^2} \right) = -2G\theta$$

$$\text{Let } B = -\frac{G\theta}{\frac{1}{h^2} + \frac{1}{b^2}} = -\frac{G\theta b^2 h^2}{b^2 + h^2}$$

then $\phi(x, y)$ is the stress function

Stress: $\left\{ \begin{array}{l} \sigma_{xx} = \frac{\partial \phi}{\partial y} = \frac{2B}{b^2} \cdot y \\ \sigma_{yy} = -\frac{\partial \phi}{\partial x} = -\frac{2B}{h^2} \cdot x \end{array} \right.$

Find θ :

$$T = 2 \iint_A \phi \, dx \, dy \Rightarrow 2 \iint_A B \left(\frac{x^2}{h^2} + \frac{y^2}{b^2} - 1 \right) \, dx \, dy$$

$$\Rightarrow 2B \left[\frac{1}{h^2} \iint_A x^2 \, dx \, dy + \frac{1}{b^2} \iint_A y^2 \, dx \, dy - \iint_A 1 \, dx \, dy \right]$$

Define $I_x = \iint_A y^2 \, dx \, dy$, $I_y = \iint_A x^2 \, dx \, dy$

$$\Rightarrow T = 2B \left(\frac{I_y}{h^2} + \frac{I_x}{b^2} - A \right)$$

Since $I_x = \frac{\pi b^3 h}{4}$ $I_y = \frac{\pi b h^3}{4}$

$$A = \pi b h$$

$$\Rightarrow T = -\pi b h = (\pi b^3 h^3) / (b^2 + h^2) \cdot G\theta$$

Define $J = \frac{\pi b^3 h^3}{b^2 + h^2}$

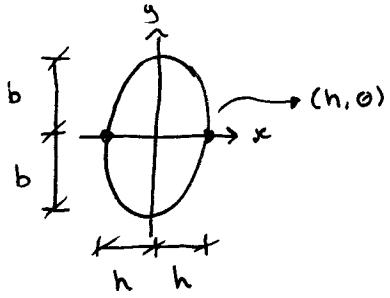
$$\Rightarrow T = GJ\theta \quad \theta = T/GJ$$

Shear Stress

$$I = \sqrt{\sigma_{zx}^2 + \sigma_{zy}^2}$$

$$= \sqrt{\left(\frac{2B}{b^2} \cdot y\right)^2 + \left(-\frac{2B}{h^2} \cdot x^2\right)^2}$$

- 1° max shear will occur at the boundary of the cross-section
- 2° max shear occurs at the boundary nearest the centroid of the cross-section



$$h < b$$

$$\sigma_{xz} = 0 ; \sigma_{zy} = -\frac{2B}{h^2} \cdot h$$

$$\sigma_{zy} = -2/h \cdot \left(-\frac{G\theta b^2 h^2}{b^2 + h^2}\right)$$

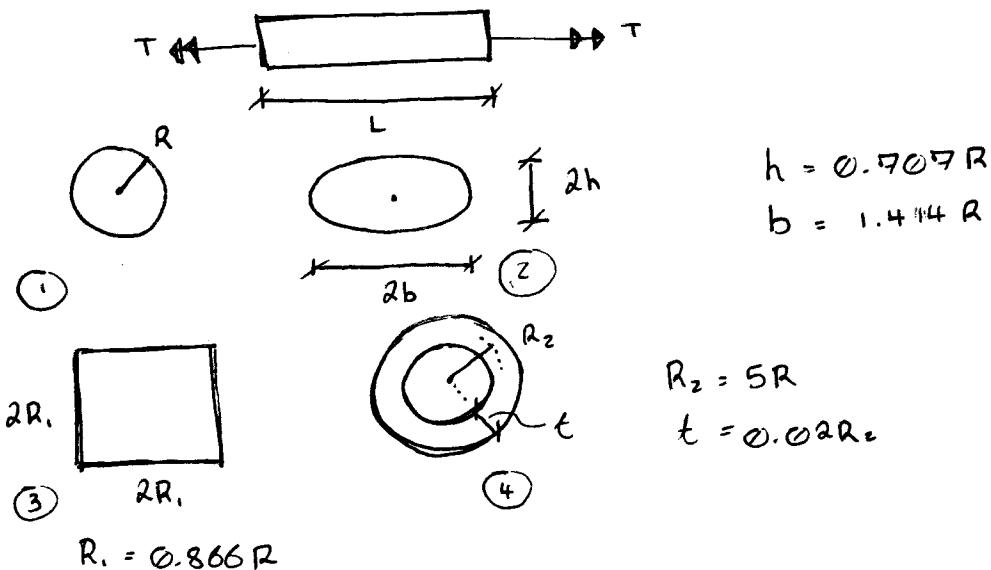
$$= G\theta \cdot \frac{b^2(2h)}{b^2 + h^2}$$

$$\Rightarrow \tau_{max} = T/J \cdot \frac{b^2(2h)}{b^2 + h^2}$$

$$= \frac{T}{\frac{\pi b^3 h^3}{b^2 + h^2}} \cdot \frac{b^2(2h)}{(b^2 + h^2)}$$

$$\Rightarrow \boxed{\tau_{max} = \frac{2T}{\pi b h^2} (h < b)}$$

Example:



$$h = 0.707 R$$

$$b = 1.414 R$$

$$R_2 = 5R$$

$$t = 0.02R_2$$

$$R_1 = 0.866 R$$

For section ③

$$J = 0.141 (2R_1)^4$$

$$I_{max} = \frac{T}{(0.208)(2R_1)^3}$$

$$\theta = T/GJ \quad \text{for all members}$$

→ Find: 1° θ for each section2° I_{max} for each section

1°)

J for each
Section

$$J_1 = \pi R^4 / 2$$

$$J_2 = \frac{\pi b^3 h^3}{b^2 + h^2} \Rightarrow J_2 = \frac{\pi (1.414 R)^3 (0.707 R)^3}{(1.414 R)^2 + (0.707 R)^2}$$

$$J_3 = 0.141 (2R_1)^4$$

$$\text{where } R_2 = 5R, \quad t = 0.02R_2$$

$$t = 0.1R$$

$$\text{then } r_1 = 4.95R, 5.05R$$

$$J_4 = \frac{\pi (5.05R^4 - 4.95R^4)}{2}$$

Comparing:

$$\textcircled{1}: J_1 = 1.571 R^4$$

$$\textcircled{2}: J_2 = 1.257 R^4$$

$$\textcircled{3}: J_3 = 1.390 R^4$$

$$\textcircled{4}: J_4 = 78.54 R^4$$

Treat as
constant

$$\theta = \frac{T}{G} \left(\frac{1}{J} \right) \Rightarrow \textcircled{4} \text{ has the largest } \theta$$

(2)

2.) Z_{max} :

$$\textcircled{1}: Z_{max,1} = \frac{T}{J_1} R = \frac{T}{1.571 R^3}$$

$$\textcircled{2}: Z_{max,2} = \frac{2T}{\pi b h^2} = \frac{2T}{\pi (1.414 R)(0.707 R)^2}$$

$$= \frac{T}{1.110 R^3}$$

$$\textcircled{3}: Z_{max,3} = \frac{T}{(0.208)(2R_1)^3} = \frac{T}{(0.208)(2 \times 886 R)^3}$$

$$= \frac{T}{(1.15) R^3}$$

$$\textcircled{4}: Z_{max,4} = \frac{T}{J_4} R_2$$

$$\Rightarrow \frac{T}{78.54 R^4} (5R) \Rightarrow \frac{T}{15.71 R^3}$$



Midterm covers Ch. 1, 2, 3